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D'APPOLONIA CONSULTING ENGINEERS INC PITTSBURGH PA  
NATIONAL DAM SAFETY PROGRAM. ALEXANDER LAKE DAM (INVENTORY NUMB--ETC(U)  
JUL 81 L D ANDERSEN

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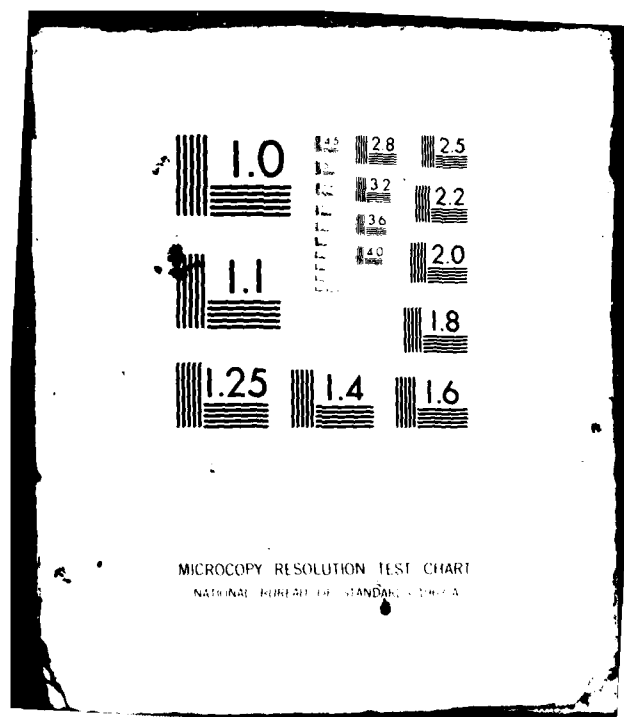
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AD A109782

SUSQUEHANNA RIVER BASIN  
**ALEXANDER LAKE DAM**

TIOGA COUNTY, NEW YORK  
INVENTORY NO. N.Y. 936

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PREPARED FOR

NEW YORK DISTRICT CORPS OF ENGINEERS  
JULY 1981

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REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
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AD-A109782		
4. TITLE (and Subtitle) Phase I Inspection Report Alexander Lake Dam Seneca River Basin, Tioga County, NY Inventory No. 839		5. TYPE OF REPORT & PERIOD COVERED Phase I Inspection Report National Dam Safety Program
6. AUTHOR(s) LAWRENCE D. ANDERSEN		7. PERFORMING ORG. REPORT NUMBER
8. PERFORMING ORGANIZATION NAME AND ADDRESS D'Appolonia Consulting Engineers, Inc. 10 Duff Road Pittsburgh, PA 15235		9. CONTRACT OR GRANT NUMBER(s) DACW51-81-C-0011
10. CONTROLLING OFFICE NAME AND ADDRESS Department of the Army 26 Federal Plaza New York District, CofE New York, New York 10281		11. REPORT DATE 14 August 1981
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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This document provides information and analysis of the physical condition of the dam. The report contains information and analysis based on visual inspection of the dam by the performing organization.  Based on the evaluation of the existing conditions, the condition of Alexander Lake Dam is considered to be fair. A minor seepage area and some wet areas are located along the downstream toe. The vertical and horizontal alignment of the dam crest is irregular. However, the		

observed conditions are not consistent to significantly affect the overall performance of the dam at this time.

The owner's representative reported that the low level outlet pipe for the embankment was plugged by concrete prior to the filling of the dam, as required by the design drawings. Therefore, the dam has no functional low level outlet facilities that could draw down the reservoir in the event of an emergency.

The spillway capacity was evaluated according to the recommended procedure and was found to pass 80 percent of the Probable Maximum Flood (PMF) without overtopping the dam and full PMF with a minor overtopping of the embankment. The spillway capacity of the dam is rated to be inadequate because the spillway cannot pass the recommended spillway design flood of full PMF without overtopping the dam.

## PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20316. The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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PHASE I INSPECTION REPORT  
 NATIONAL DAM SAFETY PROGRAM  
 ALEXANDER LAKE DAM  
 S.Y. 936  
 SEC 1.9. RD. 04a-1307  
 SUSQUEHANNA RIVER BASIN  
 TIGER COUNTY, NEW YORK

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**Page 1 Inspection Report**  
**Building and Safety Division**

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**12. Answer: D**

**Keywords:** *adolescents, delinquency, family, intervention, parents, risk factors, social support*

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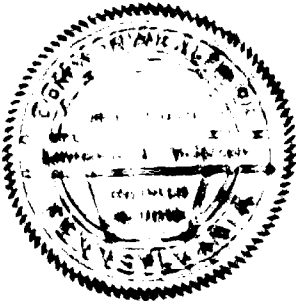
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Assessment - Alexander Lake Dam

3. The crest of the dam should be surveyed and the low spots filled to provide a uniform dam crest level. In conjunction with this work, the upstream face of the dam should be scuffed and the need for placing erosion protection should be indicated by the owner.
4. An emergency action plan should be developed including a formal warning system to alert the downstream community in the event of an emergency.
5. The dam and appurtenances should be inspected regularly and necessary maintenance should be performed.



*James P. [Signature]*

Supervisor of Dam Safety, Alaska  
State Department  
1000 North Steese Avenue, Anchorage, Alaska 99503  
Phone 581-1111, Telex 154440

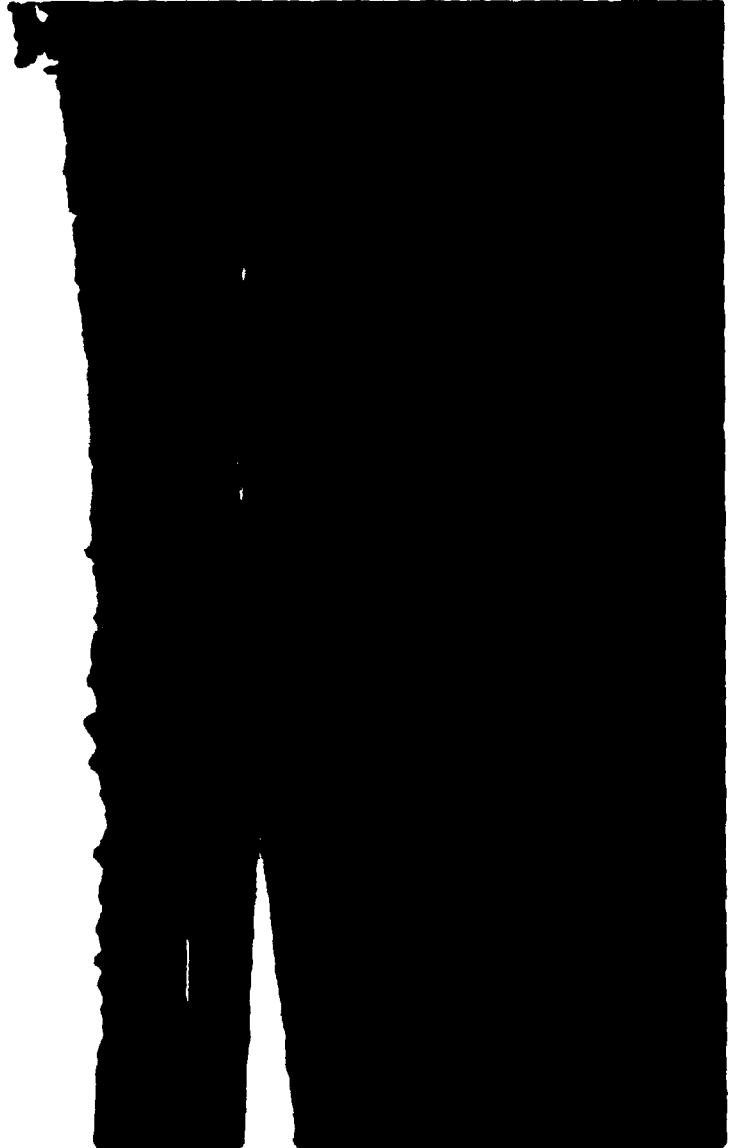
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*[Signature]*  
State Dam Safety Engineer

Date

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**PHASE I INSPECTION REPORT  
NATIONAL RRM SAFETY PROGRAM**

**ALEXANDER LAKE BOAT**

4. 1. 936

NY 100-3392

**CHICOTULANA RIVER BASIN**

**FLORA COUNTY, NEW YORK**

## SECTION 1 - PROJECT INFORMATION

1. CONFIDENTIAL

**THE UNIVERSITY OF CHICAGO**

The above charges are suggested and with are authorized by the Department of the Army, War Post Office, Office of the Inspector, to fulfill the requirements of the National War Inspection Act, Chapter 22-207.

### THE HISTORY OF THE UNITED STATES

The Program has not to duplicate the existing activities of the subject but to identify and develop new and innovative activities, particularly if they contribute towards the life and growth, and development of the subject.

[illegible]



1. 1950 年 10 月 1 日，中华人民共和国成立，标志着中国历史进入了一个新的纪元。在这一天，中国人民终于结束了长达百年的屈辱历史，实现了民族独立和国家统一。这一伟大的历史时刻，不仅改变了中国的命运，也深刻影响了世界的格局。

[illegible]

b. Location

The dam is located on an unnamed tributary of the East Branch of Omega Creek about one mile west of Newark Valley in Tioga County, New York. Plate 1 illustrates the location of the dam.

c. Size Classification

The dam is classified as a small dam based on 12-foot height and a maximum storage capacity of 158 acre-feet.

d. Hazard Classification

The dam is classified to be in the high hazard category. Approximately one mile downstream from the dam, the stream flows through a rural residential area. At least five houses are considered to be within the potential floodplain of the stream.

It is estimated that a failure of the dam would cause loss of more than a few lives and appreciable property damage in this area.

e. Ownership

The dam is owned and operated by Newark Valley Central School District, Newark Valley, N.Y. 31811, (604) 642-3221.

f. Purpose of Dam

Recreation

g. Design and Construction History

The dam was designed by Mr. Howard Ward, Consulting Engineer, from Candor, New York, in 1965. The dam was constructed under a state construction permit application dated May 7, 1965.

h. Normal Operating Procedure

The reservoir is normally maintained at the crest level of the uncontrolled primary spillway at Elevation 1263. The emergency spillway crest is located at Elevation 1264.8.

i. PERTINENT DATA

Elevations referred to in subsequent sections of the report were calculated based on field measurements assuming the primary spillway crest level to be at Elevation 1263 (USGS Datum) which was interpolated from the USGS 7.5-minute Newark Valley Court quadrangle as the normal pool level for the lake. Elevations shown in design drawings appear to be relative to an arbitrary site datum.

j. Drainage Area (acres)

263(1)

(1) Interpolated from USGS topographic map.

<b><u>b. Discharge at Dam (cfs)</u></b>	
Principal spillway at top of dam	5(estimated)
Auxiliary spillway at top of dam	1032
Total spillway capacity at top of dam	1037
<b><u>c. Elevation (USGS Datum) (feet)</u></b>	
Top of dam	1267.6
Auxiliary spillway crest	1264.8
Principal spillway crest	1263.0
<b><u>d. Reservoir (acres)</u></b>	
Surface area at top of dam	21.1
Surface area at crest of auxiliary spillway	20.0
Surface area at crest of principal spillway	19.3
<b><u>e. Storage Capacity (acre-feet)</u></b>	
Top of dam	158
Auxiliary spillway crest	108
Principal spillway crest	65
<b><u>f. Dam</u></b>	
Type	Earth embankment
Length	450 feet
Height	12 feet
Top width	9 feet
Side slopes	Downstream: 3H:1V
	Upstream: 3H:1V
Zoning	No
Impervious core	No
Cutoff	Yes
Grout curtain	No
<b><u>g. Primary Spillway</u></b>	
Type	3-foot-diameter corrugated metal pipe drop inlet
Length	9-foot perimeter
Crest elevation	1263
<b><u>h. Emergency Spillway</u></b>	
Type	Vegetated trapezoidal earth channel
Length	60 feet (as measured)
Crest elevation	1264.8
<b><u>i. Reservoir Drain</u></b>	
The dam has no functional reservoir drain facility.	

## SECTION 2: ENGINEERING DATA

### 2.1 DATA AVAILABLE

Available information was obtained from New York State Department of Environmental Conservation, Dam Safety Division files. Available information consists of three design drawings. No other information or reference to such information was located. Mr. Donald Alexander, the owner's representative, was interviewed to obtain additional information on the design and construction of the dam.

### 2.2 GEOLOGY

The Alexander Lake Dam is located in the glaciated Allegheny Plateau section of the Appalachian Plateau Province. This section is characterized as a maturely dissected plateau with the features modified by continental glaciation, including deposition of glacial till in the valleys.

The dam site is near the axis of a northeast trending anticline (approximately north 70 degrees east). The folding is gentle with the maximum dip of the limbs one to two degrees. The strata at the dam are nearly horizontal and the dip of the strata are affected locally by the folding; however, regionally, the rock strata dip south to southwest at approximately 100 to 150 feet per mile. Regional discontinuities trend approximately east-west and north-east.

The rock strata in the area consist of unconsolidated Pleistocene glacial till (Wisconsin Drift) underlain by strata of the Lower West Falls Group (Upper Devonian Age). The glacial till consists of a mixture of clay and silt with varying amounts of gravel. The glacial till is relatively thin on hilltops and slopes and thicker in the valleys. The bedrock consists of the Gardeau Formation and the Rorich Glen Shale, a thick sequence of inter-bedded very dark gray shale and thin siltstone.

The abutment slopes are relatively gentle and not susceptible to landslide slope movement.

### 2.3 SUBSURFACE INVESTIGATION

No reference was found to indicate a subsurface investigation was conducted in conjunction with the design of the dam. A note included on the design drawing shown in Plate 2 suggests that some test pits may have been excavated to classify the soils in the area.

### 2.4 EMBANKMENT AND APPURTENANT STRUCTURES

Plate 2 illustrates the typical cross section of the dam. The dam appears to consist of homogeneous fill with a central cutoff



trench. The dam was designed to have a slope of 2 horizontal to 1 vertical downstream and 3 horizontal to 1 vertical upstream. The valley cross section of the dam is included in Plate 3.

The appurtenant structures include a drop inlet primary spillway and open-channel emergency spillway located on the left abutment. Details of the primary spillway are illustrated in Plate 2.

#### 2.5 CONSTRUCTION RECORDS

No records are available on the construction of the dam. According to the owner's representative, the dam was constructed under the supervision of the design engineer.

#### 2.6 OPERATING RECORDS

No operating records are maintained for the dam. The dam is maintained by the owner's personnel.

#### 2.7 EVALUATION OF DATA

The available information does not provide any quantitative data for the assessment of structural, geotechnical and hydraulic features of the dam. The design drawings indicate the low level and primary spillway outlet pipes consist of metal pipes. Because metal pipes are subject to corrosion and failure of one of these pipes may cause distress in the embankment, concern exists as to the structural condition of the facilities. Therefore, the owner should evaluate the structural adequacy of the facilities.

The available information includes no hydrology and hydraulic analysis. Plate 4 shows the design maximum pool level. In the construction permit application to the state, the design capacity of the spillway is noted to be 454 cfs.

### SECTION 3: VISUAL INSPECTION

#### 3.1 FINDINGS

##### a. General

Visual inspections of Alexander Lake Dam were conducted on March 27 and April 30, 1981. The pool level on the dates of inspection was approximately at the primary spillway level.

##### b. Embankment

In general, the condition of the dam is considered to be fair. Field observations are illustrated in Plate 5. Two wet areas were observed along the downstream toe of the dam. No seepage flow appeared to be associated with the wet areas. A minor seepage was located on the downstream toe in an area which appeared to be the discharge channel of the low level outlet facility. The upstream slope shoreline was found to be irregular and lacked erosion protection. However, no significant erosion due to wave action was noted.

The dam crest was surveyed relative to the primary spillway crest elevation and was found to have some vertical irregularities. While the design freeboard for the dam is 4 feet, the field survey indicated freeboard ranging from 4.3 to 5.8 feet. The lowest area is at the center of the embankment. The dam crest profile according to field measurements is illustrated in Plate 6.

##### c. Primary Spillway

The primary spillway consists of a 3-foot-diameter corrugated metal pipe drop inlet structure which discharges into a 24-inch metal pipe terminating at the downstream toe of the dam. Although visible portions of the primary spillway facilities were found to be in good condition, concern exists as to the condition of the metal pipe through the embankment.

##### d. Emergency Spillway

The emergency spillway is a trapezoidal vegetated earth channel located on the left abutment. The emergency spillway channel was found to be in good condition. The grass cover is well established and adequately maintained. The approach and discharge channel were found to be free of brush and trees or debris which might pose a potential for blockage of the spillway.

##### e. Reservoir Drain

The dam does not have a functional reservoir drain pipe. The owner's representative indicated that the upstream end of the reservoir drain pipe was plugged with concrete prior to the filling of the dam.

f. Downstream Channel

Downstream channel below the primary spillway outlet pipe is an unprotected earth channel which flows parallel to the toe of the dam for approximately 100 feet and then joins the natural streambed. The channel appears to be stable in the near vicinity of the dam.

g. Reservoir

The reservoir slopes are gentle and no sign of instability was observed.

**3.2 EVALUATION**

The overall condition of the dam is considered to be fair. The following conditions were observed which require action by the owner:

1. The condition and structural adequacy of the spillway outlet pipe and reservoir drain pipe should be evaluated by the owner.
2. The crest of the dam should be surveyed and low spots filled to provide a uniform dam crest level.
3. The upstream face of the dam should be reshaped and the need for providing erosion protection should be evaluated. The wet areas and the seepage point below the toe of the dam should be periodically observed to document if significant seepage is developing.

## **SECTION 4: OPERATION AND MAINTENANCE PROCEDURES**

### **4.1 PROCEDURES**

The reservoir is normally maintained at the primary spillway crest level with excess inflow discharging through the primary spillway. The dam has no formal operating procedure.

### **4.2 MAINTENANCE OF THE DAM**

The dam is maintained by the owner's personnel. The crest and upstream and downstream faces of the dam and the emergency spillway are covered with grass and were found to be adequately maintained.

### **4.3 WARNING SYSTEM IN EFFECT**

No formal warning system exists for the dam.

### **4.4 EVALUATION**

The maintenance condition of the dam is considered to be fair. The development of a formal warning system is considered to be advisable. Further, in view of a lack of a functional low level outlet facility to drain the lake in the event of an emergency, it is recommended that the owner should develop plans to draw down the reservoir in the event an emergency.

## SECTION 5. HYDRAULIC/HYDROLOGIC

### 5.1 DRAINAGE AREA CHARACTERISTICS

The dam has a watershed of 0.4 square miles. The drainage area is comprised of woodlands and farmlands. Relief ranges from gentle to moderate.

### 5.2 ANALYSIS CRITERIA

As previously stated, Alexander Lake Dam is classified as a small dam in the high hazard category. Under the recommendations of the Federal Emergency Management Agency for evaluation of emergency spillway discharge capacity, such impoundments are required to pass overtopping at full PMP. In view of the high downstream damage potential, full PMP was selected as the spillway design flood.

The PMP inflow for the reservoir was determined using the dam safety version of the HEC-1 computer program developed by the Hydrologic Engineering Center of the U.S. Army Corps of Engineers. The data used for the computer input are presented in Appendix B. The PMP inflow hydrograph was found to have a peak flow of 1200 cfs. Computer outputs are also included in Appendix B.

### 5.3 SPILLWAY CAPACITY

The spillway facilities for the dam consist of primary and emergency spillways. The emergency spillway is a trapezoidal earth channel with a base width of 60 feet and side slopes of 3 horizontal to 1 vertical on the abutment side and about 6 horizontal to 1 vertical on the downstream side. The primary spillway is a 24-inch-diameter corrugated metal riser discharging into a 24-inch corrugated metal pipe. The PMP inflow hydrograph was routed through the reservoir and it was found that the dam can pass 90 percent of the PMP without overtopping the low spots on the crest of the dam. Because the capacity of the primary spillway is negligible compared to the emergency spillway, only the emergency spillway capacity was used in the calculations. For full PMP, it was found the low spots on the crest of the dam could be overtopped for 1.3 hours with a maximum depth of about 0.2 feet. Based on the available head relative to the low spot on the crest of the dam, capacity of the emergency spillway was calculated to be 1032 cfs. Emergency spillway rating calculations are also included in Appendix B.

### 5.4 RESERVOIR CAPACITY

The dam impounds a reservoir with a storage capacity of 63 acre-feet at the primary spillway crest level, 100 acre-feet at the emergency spillway crest level, and 150 acre-feet at the level of the low spot on the crest of the dam.

1. Statement of Assets

The following statement sets forth the assets of the Corporation as of the close of business on December 31, 1937, and shows the disposition of the same.

1. Statement of Liabilities

The following statement sets forth the liabilities of the Corporation as of the close of business on December 31, 1937, and shows the disposition of the same.

<u>Assets</u>	<u>Liabilities</u>	<u>Assets</u>
<u>(Amount)</u>	<u>(Amount)</u>	<u>(Amount)</u>
\$100	\$100	\$1
20	20	20
100	100	10

1. Statement of Income

The following statement sets forth the income of the Corporation for the year ended December 31, 1937, and shows the disposition of the same.

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1. Introduction

2. Purpose

The purpose of this document is to provide information regarding the current status of the project and to outline the objectives and scope of the work to be performed. This document is intended for the use of the project team and management.

The project is a multi-phase effort designed to develop a new system for the management of the company's resources. The project is being undertaken in order to improve the efficiency of the company's operations and to reduce the cost of its products.

3. Scope of Work

The scope of work for this project includes the development of a new system for the management of the company's resources. The project will involve the design, development, and testing of the system.

4. Organization of the Project

The project is being organized into four main phases: planning, design, development, and testing. Each phase will be managed by a different team member. The project will be completed by the end of the year.

5. Conclusion

The project is a complex and challenging task, but it is one that is well within the capabilities of the project team. The project will be completed by the end of the year.

6. References

1. The project is a complex and challenging task, but it is one that is well within the capabilities of the project team. The project will be completed by the end of the year.
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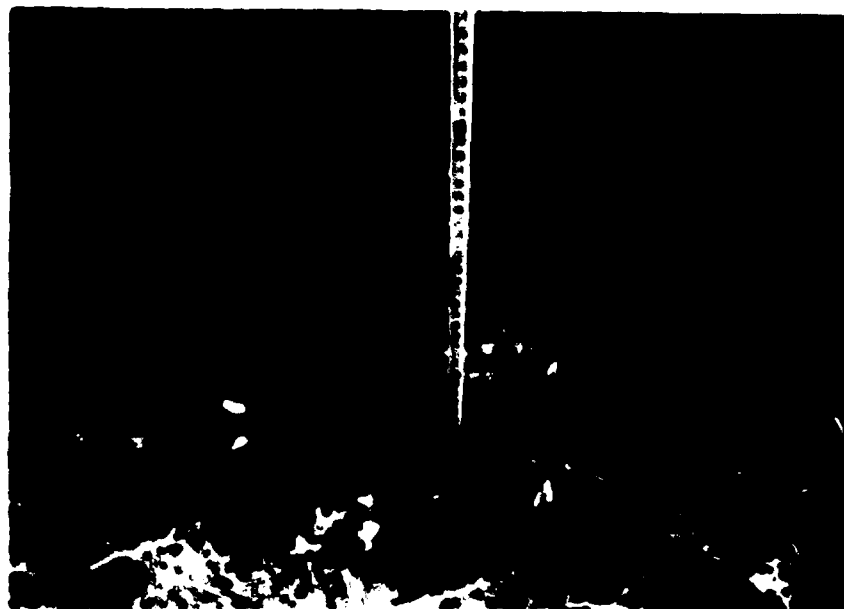
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PHOTOGRAPH NO. 2  
 View of Lake Superior from (Hutchinson) (2,000 ft. high) (approx.)



PHOTOGRAPH NO. 3  
 View of Lake Superior from (Hutchinson) (approx.)



PHOTOGRAPH NO. 5  
Primary Spillway Intake Structure



PHOTOGRAPH NO. 6  
Primary Spillway Discharge Pipe



PHOTOGRAPH NO. 1  
 Wet and Swampy Area Along Sea



PHOTOGRAPH NO. 2  
 Downstream Residential Area  
 (approximately 1.0 mile downstream)

APPENDIX B  
VISUAL INSPECTION CHECKLIST

**APPENDIX B  
VISUAL INSPECTION CHECKLIST**

**1) Basic Data**

**a. General**

Name of Dam Alexander Low Dam  
 Fed. I.D. # N.Y. 936 SEC Dam No. 004-1397  
 River Basin Susquehanna River Basin  
 Location Town Nevers Valley County Franklin  
 Section Name Structure  
 Division of Food Branch Damage Control  
 Latitude (N) 42° 12' 0" Longitude (W) 79° 12' 0"  
 Type of Dam Roller  
 Height Category High  
 Date(s) of Inspection March 27 and April 20, 1961  
 Weather Conditions Partly cloudy, temp 40's and 50's  
 Reservoir Level at Time of Inspection Normal Pool 21, 1293.0<sup>(1)</sup>  
(1293.0 feet)

b. Engineers with Dam License: Lawrence Anderson, P.E., James Fowler,  
P.E., Robert Scott, P.E., William (Bud) P.E., and Arthur Smith  
 Persons interviewed (Give full name, address & phone No.):  
(1) Mr. William Frederickson, Supervisor of Schools,  
Nevers Valley Central School District, Nevers Valley,  
P.A. 19011 (944) 942-3721  
(2) Owner's Representative: Mr. Donald Alexander

<sup>(1)</sup> Elevation measured at Frank 19011 - 5-07111 - Nevers Valley, New York State Route 14, Road 19011, at normal pool level

2. History

Date Construction 1965 Date(s) Reconstruction N/A

Designed by George O. Ward, P.E., Consulting Engineer, Canton, N.Y.

Constructed by W. Fred Jones (a local contractor)

Owner South Valley Coal and Bitum. District

3. Inspection

a. Check on Construction

(1) Inspection of Work Good

(2) Check on Design Noted some minor off design

(3) Check on Construction None

(4) Check on Construction None

(5) Check on Construction None

b. Remarks

(1) Work on the structure continued, see page 9 for info

on the structure

(2) Work on the structure continued, structure

(3) Work on the structure None

on the structure

(4) Work on the structure None

on the structure

c. Design Notes

(1) Design Notes See page 9 for info

(2) Design Notes None

on the structure

(3) Design Notes None

on the structure



1) Chlorine residual chlorine residual chlorine

2) Free Chlorine residual chlorine residual chlorine

3. Chlorine residual chlorine

1) Chlorine residual chlorine residual chlorine

Chlorine residual chlorine residual chlorine

2) Chlorine residual chlorine residual chlorine

3) Chlorine residual chlorine residual chlorine

4) Chlorine residual chlorine residual chlorine

5) Chlorine residual chlorine residual chlorine

6) Chlorine residual chlorine residual chlorine

7) Chlorine residual chlorine residual chlorine

8) Chlorine residual chlorine residual chlorine

9) Chlorine residual chlorine residual chlorine

10) Chlorine residual chlorine residual chlorine



2) Drumhead

- a. Slope Good to excellent
- b. Evidence of erosion, no traffic problems
- c. Structural conditions which affect flow

3) Long Distance of Dam

- a. Distance from dam to of dam, highway, etc. at least  
five miles and this distance
- b. Damage, Structural Good
- c. Evidence of erosion beyond toe of dam None
- d. Condition of Drumhead Channel fair. A small debris  
stream.

4) Spillways (including Discharge Conveyance Channel)

- in good condition.
- a. General Service Spillway: 2-foot-diameter OVP pipe  
discharges into a 24-inch metal pipe.  
Auxiliary Spillway: Vegetated earth channel  
on left abutment.
- b. Condition of Service Spillway Good. Trash rack vulnerable  
to blockage by debris.

1. Condition of Discharge (Emergency Channel) Good

2. Condition of Discharge (Emergency Channel) Discharge is as  
described in the above report. The discharge  
is as follows:

3. Emergency Report

Type Emergency Channel Channel Channel

Location Channel Channel Channel Channel

Time Channel Channel Channel Channel

Time Channel Channel Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

Physical Condition (Describe) Channel Channel

2) 25.10.1944

• 1960-1961 1962-1963 1964-1965 1966-1967 1968-1969 1970-1971 1972-1973 1974-1975 1976-1977 1978-1979 1980-1981 1982-1983 1984-1985 1986-1987 1988-1989 1990-1991 1992-1993 1994-1995 1996-1997 1998-1999 2000-2001 2002-2003 2004-2005 2006-2007 2008-2009 2010-2011 2012-2013 2014-2015 2016-2017 2018-2019 2020-2021 2022-2023 2024-2025 2026-2027 2028-2029 2030-2031 2032-2033 2034-2035 2036-2037 2038-2039 2040-2041 2042-2043 2044-2045 2046-2047 2048-2049 2050-2051 2052-2053 2054-2055 2056-2057 2058-2059 2060-2061 2062-2063 2064-2065 2066-2067 2068-2069 2070-2071 2072-2073 2074-2075 2076-2077 2078-2079 2080-2081 2082-2083 2084-2085 2086-2087 2088-2089 2090-2091 2092-2093 2094-2095 2096-2097 2098-2099 2100-2101 2102-2103 2104-2105 2106-2107 2108-2109 2110-2111 2112-2113 2114-2115 2116-2117 2118-2119 2120-2121 2122-2123 2124-2125 2126-2127 2128-2129 2130-2131 2132-2133 2134-2135 2136-2137 2138-2139 2140-2141 2142-2143 2144-2145 2146-2147 2148-2149 2150-2151 2152-2153 2154-2155 2156-2157 2158-2159 2160-2161 2162-2163 2164-2165 2166-2167 2168-2169 2170-2171 2172-2173 2174-2175 2176-2177 2178-2179 2180-2181 2182-2183 2184-2185 2186-2187 2188-2189 2190-2191 2192-2193 2194-2195 2196-2197 2198-2199 2200-2201 2202-2203 2204-2205 2206-2207 2208-2209 2210-2211 2212-2213 2214-2215 2216-2217 2218-2219 2220-2221 2222-2223 2224-2225 2226-2227 2228-2229 2230-2231 2232-2233 2234-2235 2236-2237 2238-2239 2240-2241 2242-2243 2244-2245 2246-2247 2248-2249 2250-2251 2252-2253 2254-2255 2256-2257 2258-2259 2260-2261 2262-2263 2264-2265 2266-2267 2268-2269 2270-2271 2272-2273 2274-2275 2276-2277 2278-2279 2280-2281 2282-2283 2284-2285 2286-2287 2288-2289 2290-2291 2292-2293 2294-2295 2296-2297 2298-2299 2300-2301 2302-2303 2304-2305 2306-2307 2308-2309 2310-2311 2312-2313 2314-2315 2316-2317 2318-2319 2320-2321 2322-2323 2324-2325 2326-2327 2328-2329 2330-2331 2332-2333 2334-2335 2336-2337 2338-2339 2340-2341 2342-2343 2344-2345 2346-2347 2348-2349 2350-2351 2352-2353 2354-2355 2356-2357 2358-2359 2360-2361 2362-2363 2364-2365 2366-2367 2368-2369 2370-2371 2372-2373 2374-2375 2376-2377 2378-2379 2380-2381 2382-2383 2384-2385 2386-2387 2388-2389 2390-2391 2392-2393 2394-2395 2396-2397 2398-2399 2400-2401 2402-2403 2404-2405 2406-2407 2408-2409 2410-2411 2412-2413 2414-2415 2416-2417 2418-2419 2420-2421 2422-2423 2424-2425 2426-2427 2428-2429 2430-2431 2432-2433 2434-2435 2436-2437 2438-2439 2440-2441 2442-2443 2444-2445 2446-2447 2448-2449 2450-2451 2452-2453 2454-2455 2456-2457 2458-2459 2460-2461 2462-2463 2464-2465 2466-2467 2468-2469 2470-2471 2472-2473 2474-2475 2476-2477 2478-2479 2480-2481 2482-2483 2484-2485 2486-2487 2488-2489 2490-2491 2492-2493 2494-2495 2496-2497 2498-2499 2500-2501 2502-2503 2504-2505</

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SECRET

**\* \* \***

• **Depression =** frustration, guilt, shame, loss

1. What is the purpose of the document?

### 4. Structure of Evidence

- 1. Introduction 1/1
- 2. Background 1/1
- 3. Methodology 1/1
- 4. Results and Discussion 1/1
- 5. Conclusion 1/1
- 6. References 1/1
- 7. Appendix 1/1
- 8. Summary 1/1
- 9. Final Report 1/1



1944年10月

1944年10月10日



**APPENDIX A**  
**ANNUAL REPORT OF THE COMMISSIONER OF THE GENERAL LAND OFFICE**  
**FOR THE YEAR 1937**

**LAND ACQUISITION**

	<u>Acquired by</u> <u>(Acres)</u>	<u>Acquired by</u> <u>(Acres)</u>	<u>Acquired by</u> <u>(Acres)</u>
1) By purchase	1,234.5	54.4	1,288.9
2) By gift (State-land)	1,234.5	54.4	1,288.9
3) By gift (Private)	1,234.5	54.4	1,288.9
4) By gift (Federal)	1,234.5	54.4	1,288.9
5) By gift (Other)	1,234.5	54.4	1,288.9

1) By purchase (State-land) (Acres) 1,234.5  
 2) By gift (State-land) (Acres) 54.4

**DISPOSITION**

	<u>Disposition</u> <u>(Acres)</u>
1) By purchase	1,234.5
2) By gift (State-land)	1,234.5
3) By gift (Private)	1,234.5
4) By gift (Federal)	1,234.5
5) By gift (Other)	1,234.5
6) By purchase (Other)	1,234.5
7) By gift (Other)	1,234.5
8) By purchase (Other)	1,234.5

1) By purchase (State-land) (Acres) 1,234.5

Dam Alexander Lake Dam  
 Crest Elevation 1267.0 (measured low spot)  
 Type Earth  
 Width 9 feet Length 420 feet  
 Spillways Drop inlet and vegetated earth channel  
 Location Drop inlet: Near left abutment; Earth channel: Left  
abutment

SPILLWAY

SERVICE		AUXILIARY
<u>1267.0</u>	Elevation	<u>1264.8</u>
<u>Drop inlet</u>	Type	<u>Vegetated Earth Channel</u>
<u>2-face diameter</u>	Width	<u>60 feet</u>
<u>Type of Control</u>		
<u>Uncontrolled</u>	Uncontrolled	<u>Uncontrolled</u>
<u>Controlled</u>		
<u>N/A</u>	Type (Flashboards, gate)	<u>N/A</u>
<u>N/A</u>	Number	<u>N/A</u>
<u>N/A</u>	Size/Length	<u>60 feet wide</u>
	Invert Material	<u>Vegetated Earth</u>
	Anticipated Length of operating service	<u>Unknown</u>
<u>70 ±</u>	Chute Length	<u>N/A</u>
<u>1 ± feet</u>	Height Between Spillway Crest and Approach Channel Invert (Weir Flow)	<u>2 ± feet</u>

**Hydro-meteorological Gages.**

Type: None

Location: N/A

**Records:**

Date - N/A

Max. Reading - N/A

**FLOODWATER CONTROL SYSTEM.**

Warning System: None

Method of Controlled Releases (Automatic):

None

Observed area \_\_\_\_\_

Observed Area Description Characteristics

Contour lines & type \_\_\_\_\_

Base area & location \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area & type \_\_\_\_\_

Observed area

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

Observed area & type (including all observed characteristics and type of observed area) \_\_\_\_\_

SECRET  
CONFIDENTIAL AND PROPRIETARY

SECRET

~~SECRET~~

• • • • •

[illegible]

1. CONFIDENTIAL - This document contains information which is exempt from public release under the Freedom of Information Act, 5 U.S.C. 552, because it is information the disclosure of which is exempt under 5 U.S.C. 552(b)(7)(C).

[illegible]



[illegible]



# SUMMARY OF DAM SAFETY ANALYSIS

NO	1	.....	ELEVATION STORAGE OUTLET	INITIAL VALUE 1261.00 0.00 0.00	SPILLWAY CREST 1263.00 0.00 0.00	TOP OF DAM 1267.00 150. 1037.00	TIME OF MAX OUTFLOW HOURS	TIME OF CALCULATED SUBMERGENCE
RATIO OF PPE			MAXIMUM RESERVOIR W.S.-ELEV	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	MAXIMUM OUTFLOW HOURS		
.25			1403.00	110.	172.	0.00	0.00	0.00
.30			1260.13	127.	113.	0.00	0.00	0.00
.40			1260.47	134.	443.	0.00	0.00	0.00
.50			1260.75	140.	701.	0.00	0.00	0.00
.60			1267.04	150.	937.	0.00	0.00	0.00
.70			1267.75	155.	990.	0.00	0.00	0.00
.80			1267.00	159.	1042.	0.00	0.00	0.00
.90			1267.00	163.	1042.	0.00	0.00	0.00
1.00			1267.04	163.	1042.	0.00	0.00	0.00

OVERTOPPING ANALYSIS SUMMARY

# D'APPOLONIA

CONSULTING ENGINEERS INC

By WKS Date 5/2/81 Subject ALEXANDER ROAD DAM Sheet No. 1 of 1  
 Chkd. By WTC Date 5/7/81 NY 86A-3392 Proj. No. 80-778-01

## SPILLWAY CAPACITY

Ref: "Design of Small Dams", C.E. 503

$$V_c = \sqrt{\left(\frac{b + 8d_c}{8 + 2.2d_c}\right) d_c g} \quad (\text{Eq. 1})$$

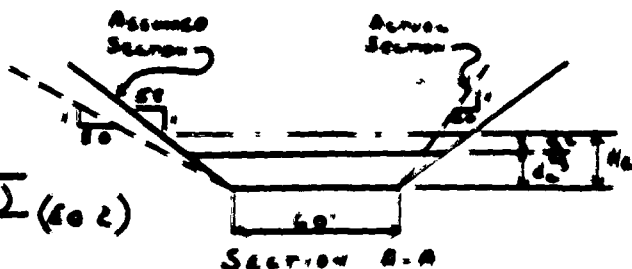
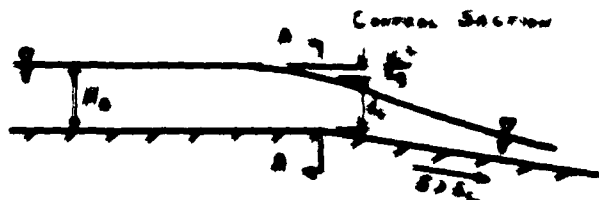
$$H_c = d_c + \frac{H^3}{2g} = d_c + \left(\frac{H^3}{8g}\right) \left(\frac{1}{d_c}\right)$$

$$= \left(\frac{3H^3 + 8gd_c^2}{8g}\right) d_c$$

$$d_c = \frac{-(3H^3 + 8gd_c^2) + \sqrt{(3H^3 + 8gd_c^2)^2 + (4H^3)(100)}}{100} \quad (\text{Eq. 2})$$

$$A_c = (2d_c + b) d_c \quad (\text{Eq. 3})$$

$$Q_c = A_c V_c \quad (\text{Eq. 4})$$



Assume 1 cfs outflow from EL 1263.1 to 1264.8 due to 6" low level  
 spill outflow

Head Elevation (ft)	H <sub>a</sub> (ft)	Eq. 2 d <sub>c</sub> (ft)	Eq. 3 A <sub>c</sub> (ft <sup>2</sup> )	Eq. 4 V <sub>c</sub> (fps)	Eq. 4 Q <sub>c</sub> Spillway Capacity (cfs)
1264.8	0	0	0	0	0
1265.5	0.7	0.5	29.7	3.8	113.5
1266.0	1.2	0.8	52.0	5.0	262.7
1266.5	1.7	1.2	78.1	5.8	456.5
1267.0	2.2	1.5	105.0	6.6	692.2
1267.5	2.7	1.9	133.6	7.2	968.7
1268.0	3.2	2.2	164.1	7.8	1285.6
1268.5	3.7	2.6	196.4	8.4	1643.1
1269.0	4.2	3.0	230.5	8.9	2041.4
1270.0	5.2	3.7	304.2	9.7	2962.7
1271.0	6.2	4.5	385.2	10.5	4054.2
1272.0	7.2	5.2	473.6	11.2	5321.5

Top of Dam - EL 1267.6

**APPENDIX E**  
**PLATES**



Will report that we have had 100%

**APPROXIMATE  
WATERSHED AREA**

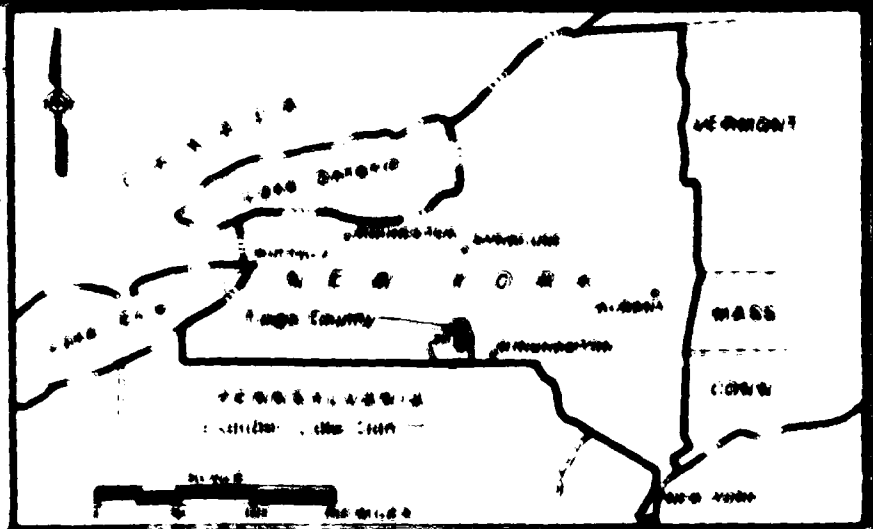
## Newark Valley

**ALEXANDER**

ALL INFORMATION CONTAINED  
HEREIN IS UNCLASSIFIED

## REFERENCE

7 3 MIN USES NEWARK VALLEY, NY QUADRANGLE  
DATED 1969. SCALE 1:24000



**KEY PLAN**

**ALEXANDER LAKE DAM**

**W I R E V A L L E Y**

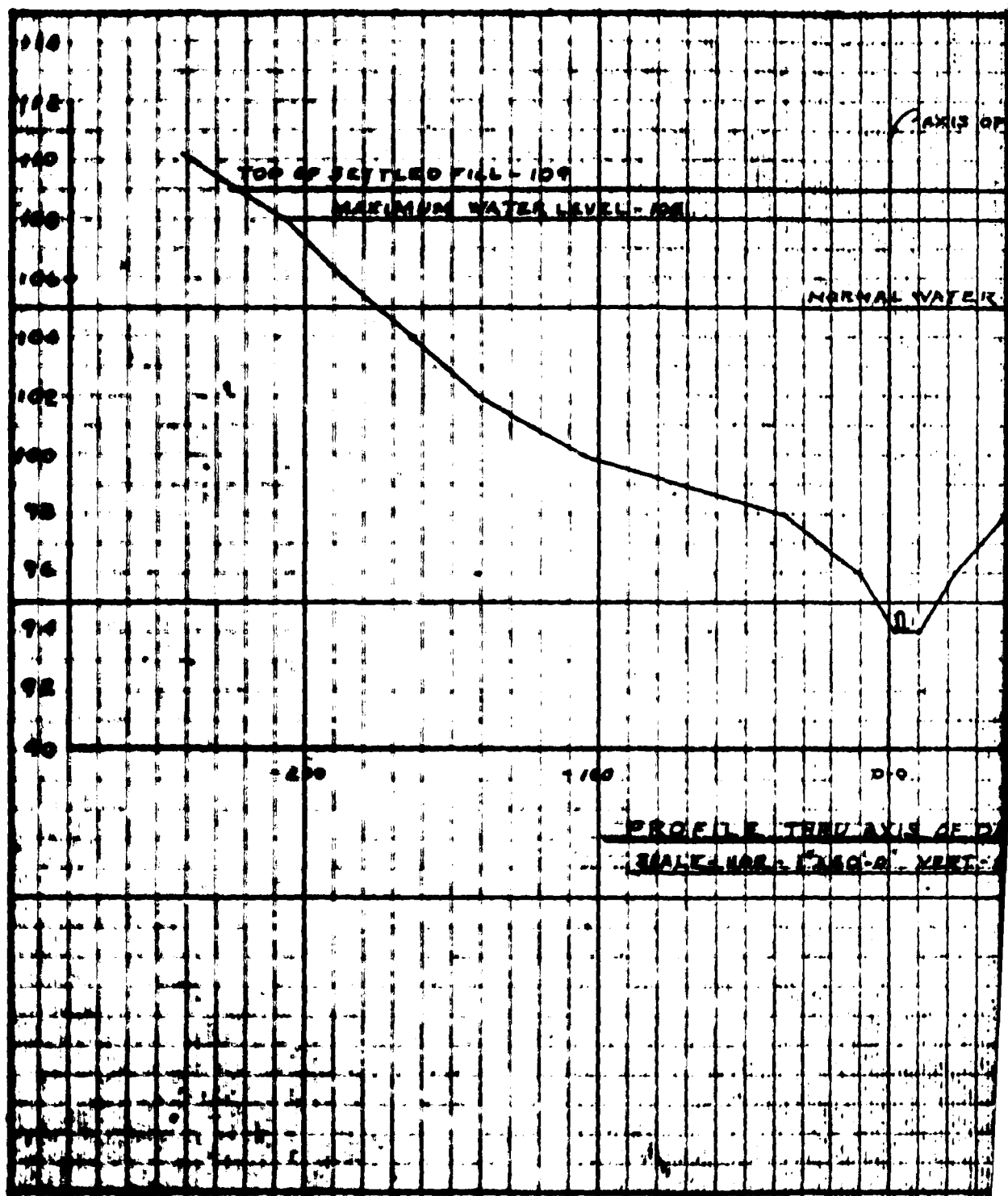
**PLATE I**  
**ALEXANDER LAKE DAM**  
**VICINITY FLOOD PLAIN & WATERSHED MAP**



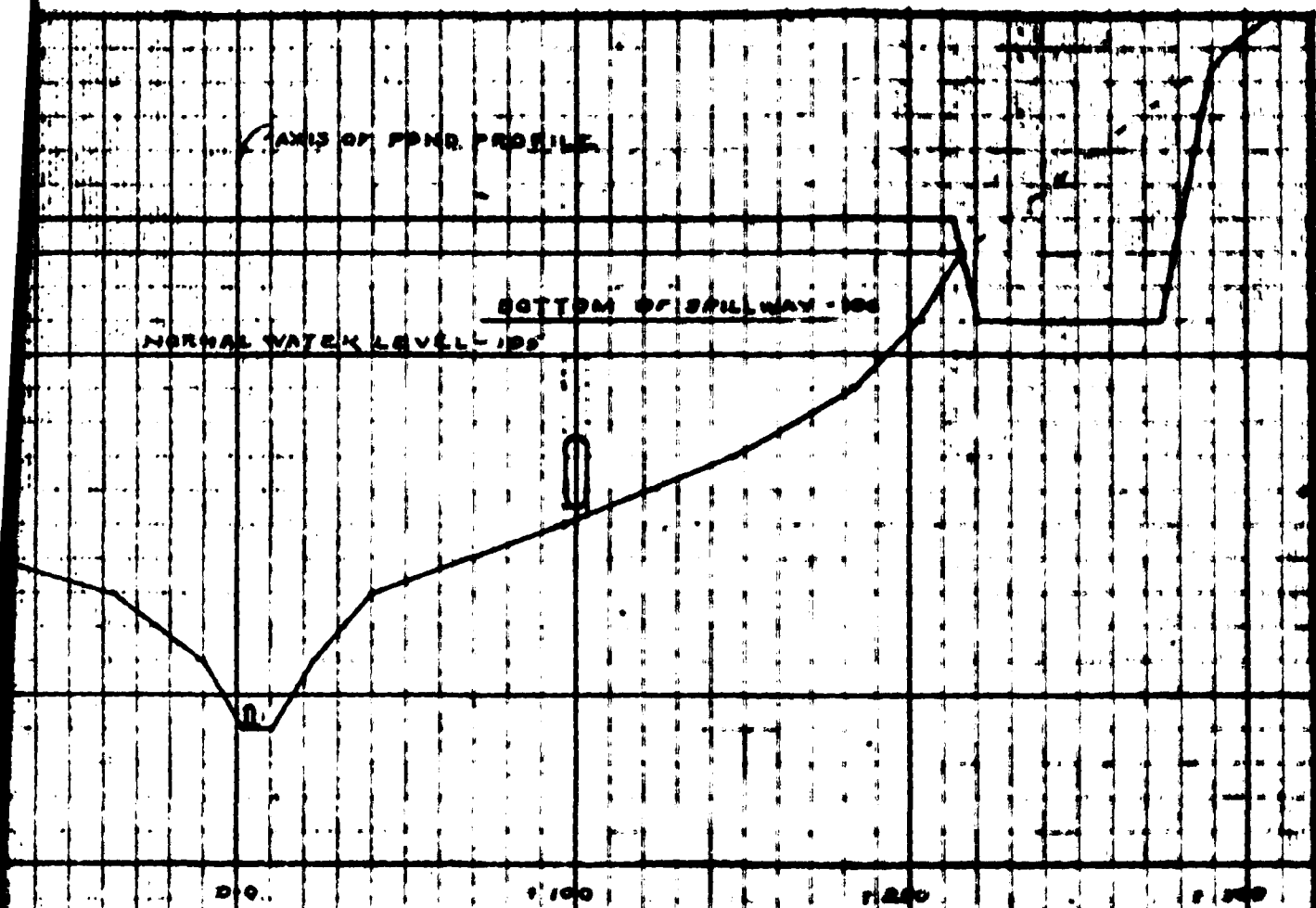
**DARTMOUTH**



# DAIPOLO NLA

[illegible]





**FILE THROUGH AXIS OF DAM**



*[Signature]*

CROSS SECTION  
INTERNATIONAL FOWLS  
NEWARK VALLEY  
SECTOR  
HOMER  
DATE

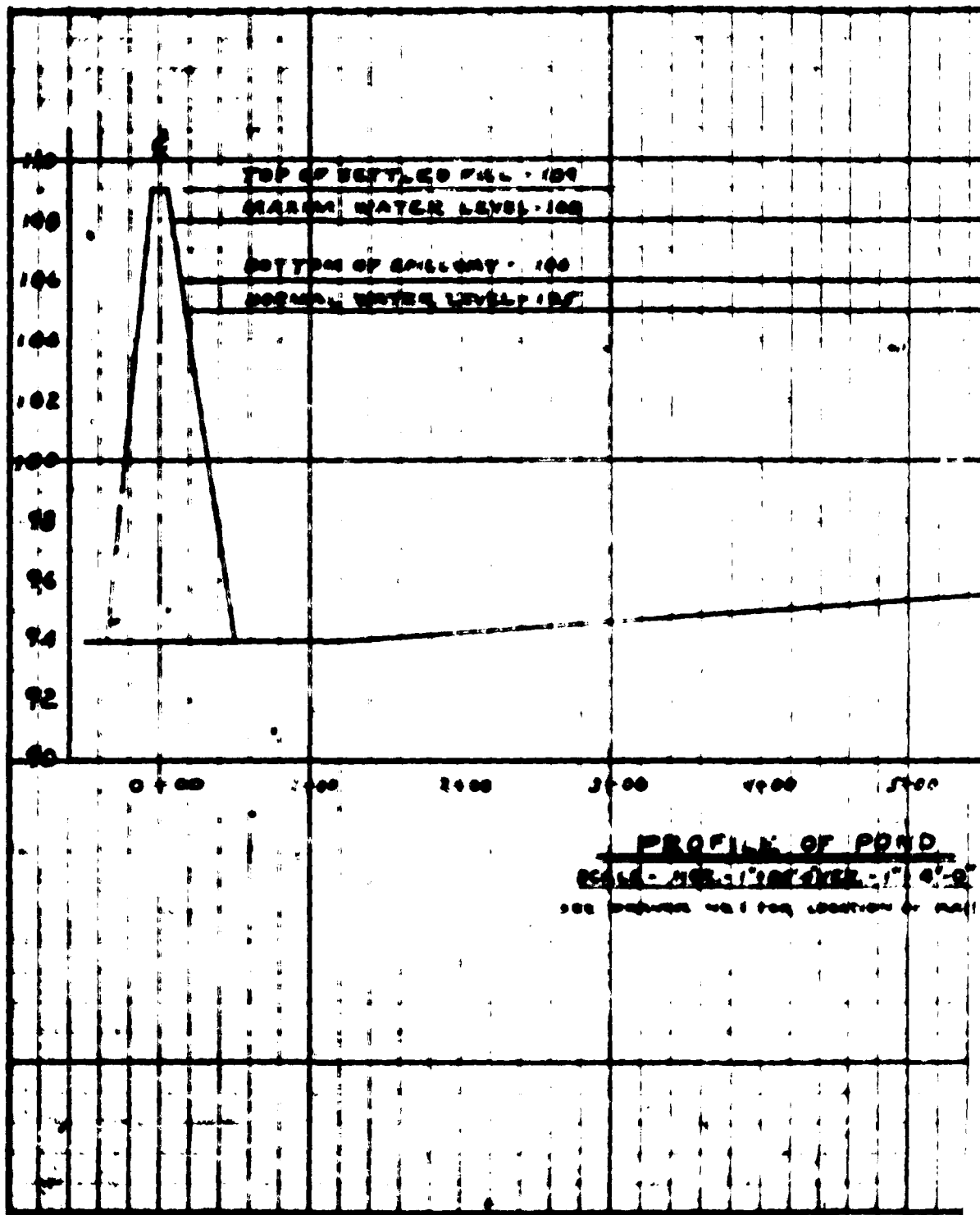
1410 P. 02 12 JAN 1974 PRACTICED  
BOSTON PRACTICED TO THE

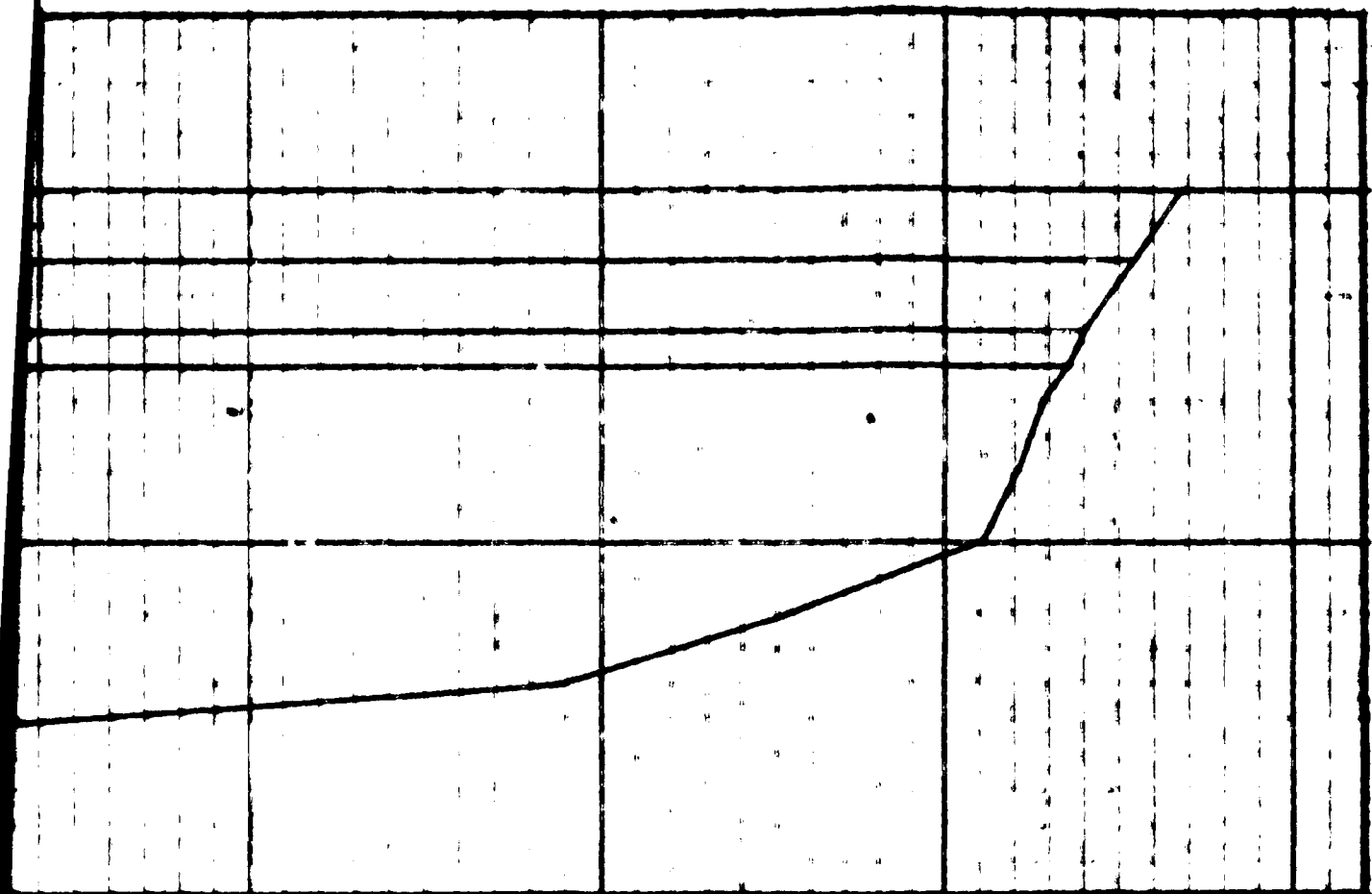
2

**PLATE 3**

# DAFTPOISONLA

DRAWN BY: [ ]  
 CHECKED BY: [ ]  
 DESIGNED BY: [ ]  
 DATE: 11/11/50  
 NUMBER: 60-776-848





400 500 600 700 800 900 1000 1100

FILE OF POND  
2-17-67-1" 4'-0"  
 Pond 1 Pond Location on Map



*Harold C. Wilson*

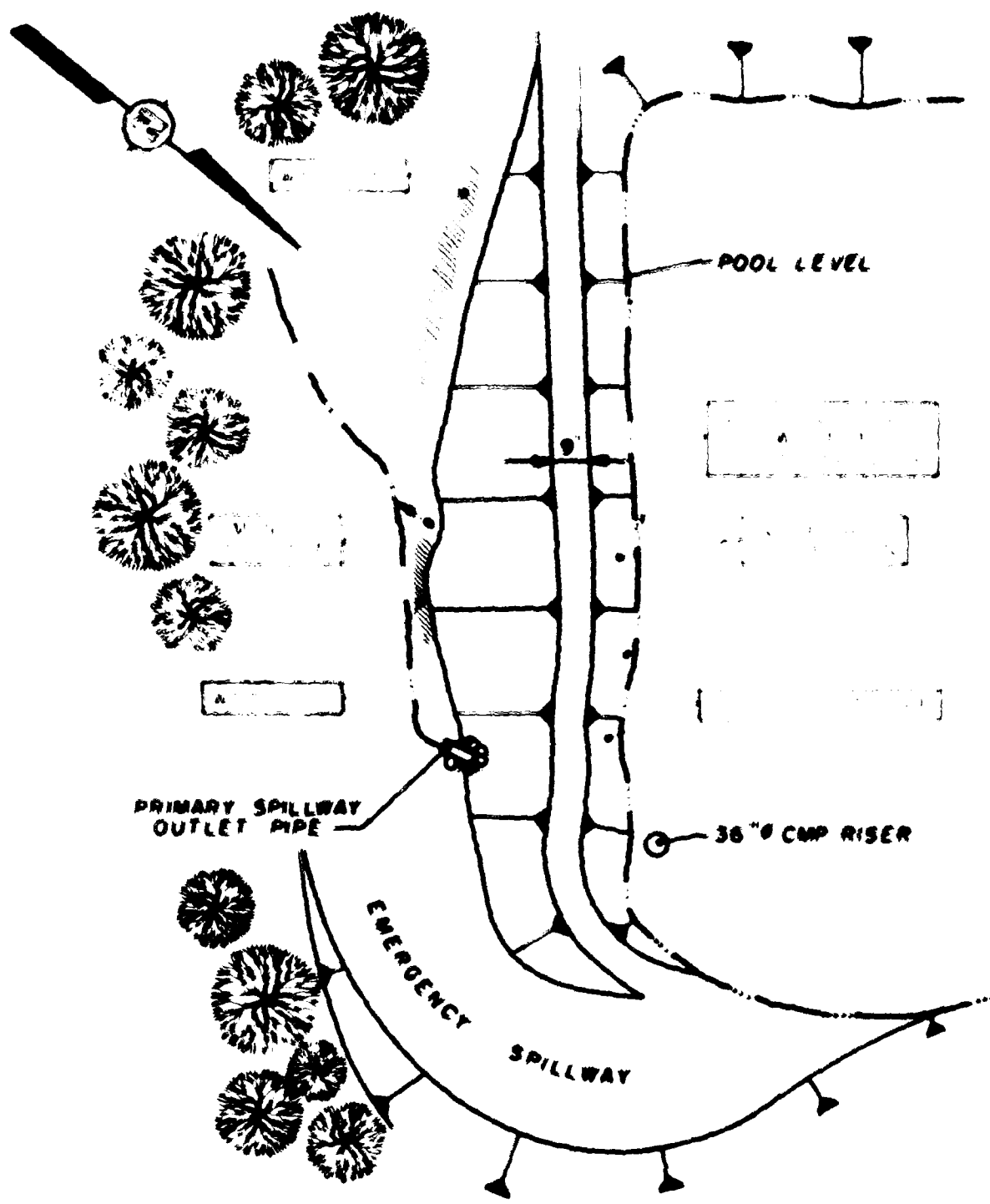
TRANSVERSE SECTION	
RECREATIONAL POND	
NEWARK VALLEY, N.Y.	
NEWARK VALLEY CENTRAL	
SCHOOL DISTRICT	
HAROLD C. WILSON	
CONSULTING ENGINEER	
CHARTERED 1952	
SCALE: 1" = 100'	DATE: 1-1-68
SHEET NO. 1 OF 2	

*2*

PLATE 4

**DAIYONIA**

DRAWN BY: [ ] CHECKED BY: [ ] DATE: [ ]  
 DRAWING NUMBER: 80-778-A-1



**NOTE**  
 POOL LEVEL AT DATE OF  
 INSPECTION PRIMARY  
 SPILLWAY CREST

**PLATE 5**  
 ALEXANDER LAKE DAM  
 GENERAL PLAN  
 FIELD INSPECTION NOTES  
 FIELD INSPECTION DATE: MAR. 27, 1981

**DAVIDMANLA**

NOT TO SCALE

**APPENDIX F**  
**GEOLOGY MAP**

DRAWN BY  
JCS  
CHECKED BY  
JCS  
DATE  
6-25-77  
DRAWING NUMBER  
80-778-A3

Beebe Lake Dam

Jennings Pond Dam

Nanticoke Creek Watershed Project  
Floodwater Retarding Dam Site 9-C

Alexander Lake Dam

Nanticoke Creek Watershed Project  
Floodwater Retarding Dam Site 7-B

Pelto Dam

Ed Pytkos Dam



GEOLOGY MAP

REFERENCE

GEOLOGIC MAP OF NEW YORK, FINGER LAKES SHEET  
DATED 1970, SCALE 1:250,000

**DAPPOLONIA**

DRAWING NUMBER 80-778-A6  
 SCALE 1"=250,000'  
 DATE 12/7/71  
 BY JCS  
 CHECKED BY JCS  
 APPROVED BY JCS

## LEGEND

### CANADIAN GROUP

800-1200 ft (240-370 m.)

- Dr. Machias Formation—shale, siltstone, Rushford Sandstone, Canadensis, Canisteo, and Hume Shales, Canadensis Sandstone, South Wales and Dunkard Shales. In Pennsylvania: Tonawanda Formation—shale, sandstone

### JOHN GROUP

300-700 ft (90-210 m.)

- Dr. Wacey Formation—sandstone, shale, Haver and Poe Creek Shales

### WEST FALLS GROUP

1100-1600 ft (340-490 m.)

- Dr. Runda Formation—sandstone, shale  
 Dr. West Hill and Gardener Formations—shale, siltstone, Raricks Glen Shale, upper Beers Hill Shale, Grimes Siltstone  
 Dr. Lower Beers Hill Shale, Dunn Hill, Millport and Maryland Shales  
 Dr. Runda Formation—sandstone, shale, West Hill Formation—shale, siltstone, Corning Shale  
 Dr. "New Millers" Formation—sandstone, shale  
 Dr. Gardener Formation—shale, siltstone, Raricks Glen Shale  
 Dr. Shale Mountain Formation—sandstone, shale, conglomeration  
 Dr. Beers Hill Shale, Grimes Siltstone, Dunn Hill, Millport, and Maryland Shales

### SONYEA GROUP

200-1000 ft (60-300 m.)

- Dr. In west: Canadensis and Middlesex Shales  
 In east: Rye Point Shale, Rock Stream ("Enfield") Siltstone, Pulteney, Sawmill Creek, Johns Creek, and Montour Shales

### GENESSEE GROUP AND TULLY LIMESTONE

200-1000 ft (60-300 m.)

- Dr. West River Shale, Genesee Limestone, Penn Yan and Genesee Shales, all except Genesee replaced eastwardly by Onondaga Formation—shale, siltstone and Sherburne Siltstone  
 Dr. Onondaga Formation—shale, sandstone  
 Dr. Onondaga Formation—shale, siltstone  
 Dr. Tully Limestone

GEOLOGY MAP LEGEND

**D'APPOLONIA**

#### REFERENCE

GEOLOGIC MAP OF NEW YORK, FINGER LAKES SHEET  
 DATED 1970, SCALE 1"=250,000'